My Research or the Project of the MSs in Computer Aided Engineering which was done in 1994 Coventry Polytechnic or at the moment Coventry University for the Rover Group Car Company in England, United Kingdom.

MSc Project/Research

Optimization of cost and weight efficiency of Rover "World" Front Seat Design in support of R\X Programme.

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Supervisors, Mr. Steve Jerrams, Dr. Ray Jones

Summary

The desire for a weight reduction and deflection analysis and stress analysis of a new front seat for the Rover R⁷X resulted in the necessity for Finite Element analysis.

A computer model was generated on CADDS-\$X system and the meshed geometry passed to a Harris computer running ANSYS and then transferred to SUN Workstation for further analysis.

Loading conditions to simulate the seat under rear impact by an unrestrained object during frontal collision were devised and applied to the model.

Cases of a "'\(\xi\)kg object hitting the seat at \(\xi\)m/s', \(\circ\)kg object at \(\xi\)m/s' and \(\xi\)kg object at \(\xi\)m/s' were examined. Model conditions of (a) Original thicknesses (\(\cdot\)mm, \(\cdot\)mm, \(\cdo

The Finite Element Analysis was carried out on R^TX front seat squab frame using Linear Static analysis, and Non-linear Static analysis due to material properties and large displacements. These results showed and proved the prediction that it is possible to make cost saving by reducing material thicknesses and increased in size of some holes and the flange in the model in comparison with the original design.

1. Introduction

Rover, as the case with other car world manufacturers are seeking to improve their product designs by reducing weight without impairing function. The front seat design, of which this project is concerned is an example. In addition to the usual service conditions the seat must also safely cope with crash situations. During sudden braking or a frontal impact of the vehicle unrestrained objects in the rear of the car are thrown forward and may hit the front seat. Under these conditions it is desirable that the seat does not fracture and become another projectile within the car. Nor is it desirable that the seat remains so rigid that it does not yield since the object hitting it could well be a rear seat passenger. To simulate these crashes resulting $\frac{1}{2}$ m/s deceleration were examined with objects such as $\frac{1}{2}$ kg. Also the case of $\frac{1}{2}$ m/s deceleration with objects of $\frac{1}{2}$ n/s and $\frac{1}{2}$ kg were examined. Although the failure mode will be the same in these cases the degree of failure or amount of deflection can be judged. Since it is also of interest to reduce the seat weight different metal thicknesses will be examined.

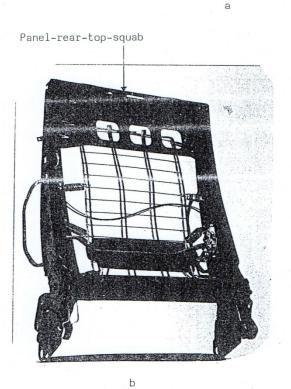
The seat design is a fairly complex $^{\text{r}}$ -D problem that lend itself to an analysis by Finite Element Methods.

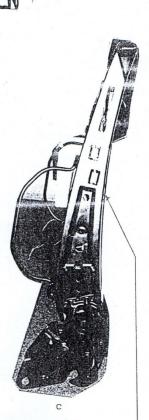
In order to produce the data for analysis, a fully three dimensional model of the seat was created on a CADDS-£X system. From this a meshed model was produced and this information transferred to SUNworkstation using an ANSYS system for analysis. In line with standard techniques only a half model was required and changes to material thickness being performed within the ANSYS program.

As with all Finite Element Work, in addition to the generation of a sensible mesh, the careful selection of loads and constraints is important. It was decided that initially the loading would be applied in the form of a pressure load at the top of the seat and the constraints would be applied rigidly at the lumbar adjustment position. Once these initial conditions had been tested the development of the model could depend on whether the results suggested changes to the model, constraints or type of analysis were required since a non-linear static analysis and linear elastic analysis were necessary.

Since there is only a limited amount of time available for the project an exhaustive investigation was not possible. Under these circumstances only a preliminary investigation would be possible but indicating areas of future work.

Panel-front -top squab





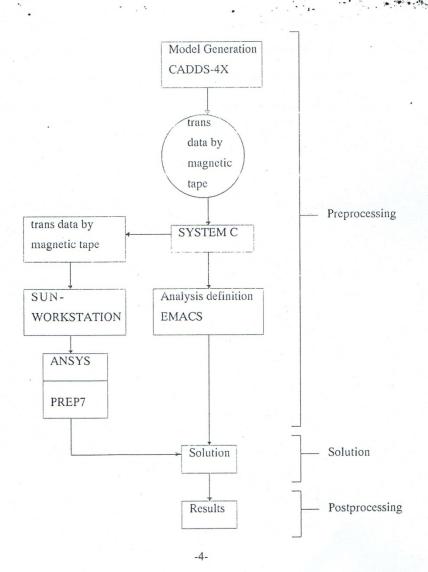
Panel-side squab-left or right hand

Figure 1 a, b and c shows three different views of the actual seat

2.0 ANSYS FINITE ELEMENT PACKAGE

To achieve the main activities of the project it was decided to use the facilities available for modelling, CADDS-4X chosen as a pre-processor and the new version of the ANSYS Package installed on a SUN-workstation for analysis and post processing.

Figure (2) shows how data transformed from CADDS-4X system to ANSYS.

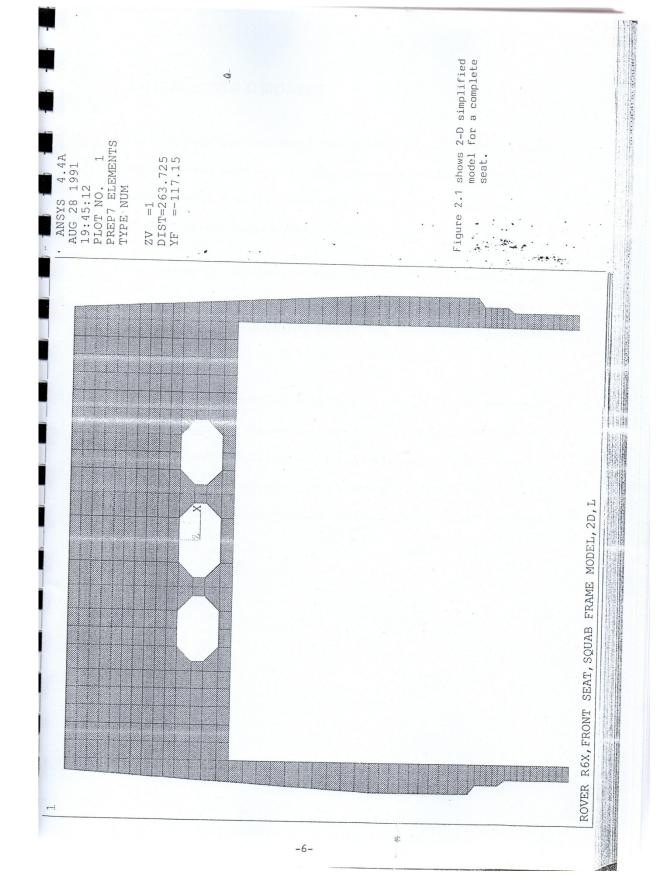


2.1 BUILDING 2-D SIMPLIFIED MODEL

The simple 2-D model of half and full seat was generated using quadrilateral shell STIF63 element. This element has six degrees of freedom at each node. This type of element has an option for variable thickness.

Key-points were defined at certain part of the model. Lines and areas were then defined and elements generated along these. The mesh size of the elements were medium. The material of the seat was mild steel to BS1449 - Part 1:1983 and the values of thickness, poisson's ratio and modulus of elasticity were defined. The half model was for linear static analysis and the full model was intended for dynamic linear analysis (see Appendix A). Pressure load was applied to the forward direction as a rear impact by an unrestrained object during a sudden braking.

These models were an attempt to allow for the creation of a more complex model.

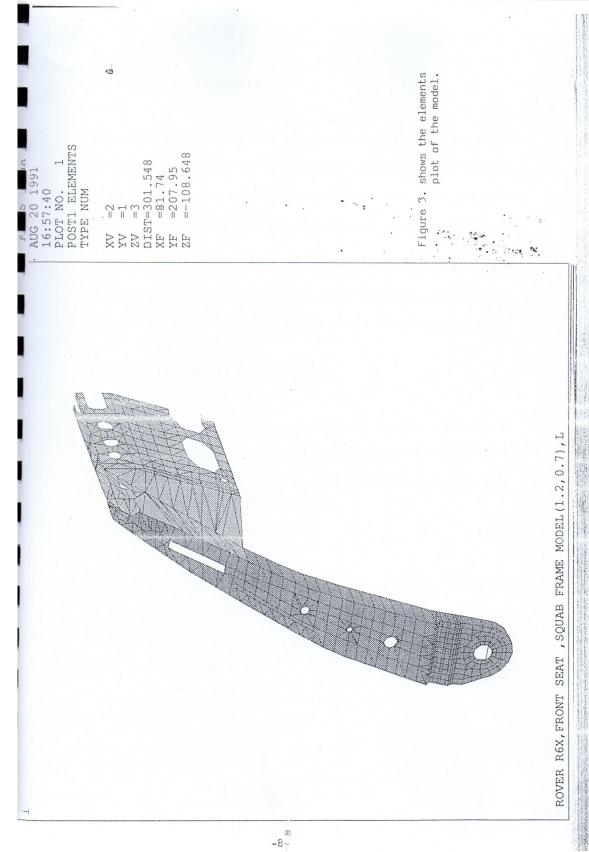


3.0 BUILDING 3-D MODELS

Due to the very complex geometry of the seat a large amount of project time would require to be spent modelling the seat to get as accurate as the dimensions shown on the original drawings. There was insufficient time to go for modelling complete seat, since only Linear and Non-linear analysis were required.

The 3-D model of one half of the seat was generated using CADDS-4X as a pre-processor i.e. a wireframe model was described in terms of points and lines, grid points and elements then were generated along these by using computervision finite element manual.

Element type used was quadrilateral shell STIF63 element. This element has six degree of freedom at each node, translation in the nodal x, y and z directions and rotations about the nodal x, y, and z axes. The quadrilateral shell has an option for variable thicknesses. The data of this model was then successfully transferred to ANSYS pre-processor where material properties, constraints and loads were added. Figure (3) shows the elements plot of the model.



3.1 ORIGINAL CONDITION

This model is the main model which was generated using CADDS-4X and ANSYS package. This model was built up using the original thicknesses of the seat as it was designed. The thicknesses were (J.2 mm) for panel side squab and (0.7mm) for front and rear panel top squab. (see Appendix B). Fig.3.1 shows plot of two different thicknesses.

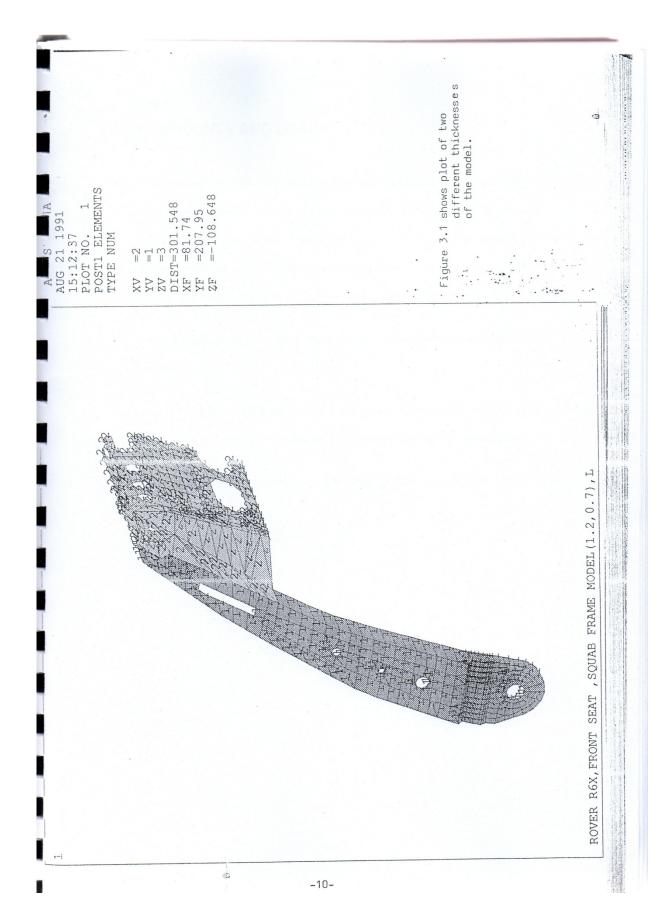
The material of the seat was mild steel to BS1449 - Part 1:1983 as stated on the drawing HFA10109 (squab frame). The mesh size of the elements were medium and fine mesh used on the area of most interest where the lumbar support bracketry restraint.

3.2 DEVELOPMENT CONDITION

This model has the same geometry of the original one but it was developed to the thicknesses of (1.1mm) for panel side squab and (0.6mm) for front and rear panel top squabs. The material of the seat was the same as the material of original condition.

3.3 PRODUCTION CONDITION

This model was presumed as a production of the seat model, again it has exactly the same geometry of the above conditions. The thicknesses of this condition were (1.0mm) for panel side squab and (0.5mm) for front and rear panel top squabs.



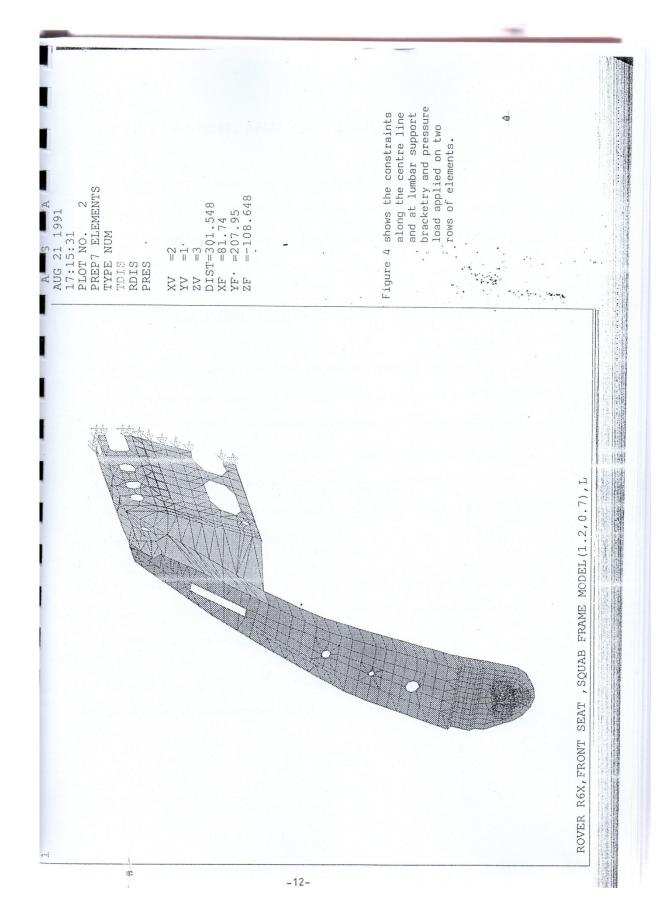
4.0 RESTRAINTS AND LOADS

After transferring the data successfully from the CADDS-4X to the ANSYS pre-processor where material properties, constraints and loads were added. The seat was constrained along the centre line in the z-direction, and at lumbar support bracketry in all directions where the permanent fixing of full seat at both sides would occur. Loads were applied in a form of elements pressure as a rear impact (see Appendix D).

The type of loading applied was 51 kg object at 4 m/s^2 for Linear static analysis on all three conditions.

The cases of 34 kg object hitting the seat at 4 m/s^2 and 51 kg object at 4 m/s^2 , 68 kg at 4 m/s^2 and 84 kg at 4 m/s^2 for Non-Linear static analysis on original condition only.

For 3/4 of load and half load on three conditions see tables of linear analysis in Appendix (G).



5.0 LINEAR STATIC ANALYSIS RESULTS

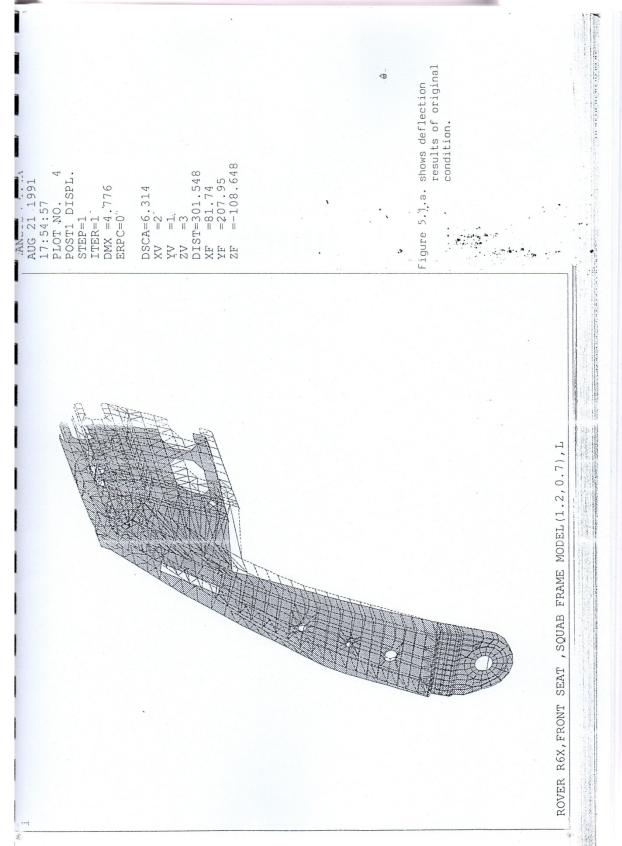
The results of the three models (original, development and production conditions) were obtained. These values represent deflections, principal stresses and Von Mises equivalent stress in maximum and minimum values. For SIG2 and SIG3 see the tables in Appendix (G).

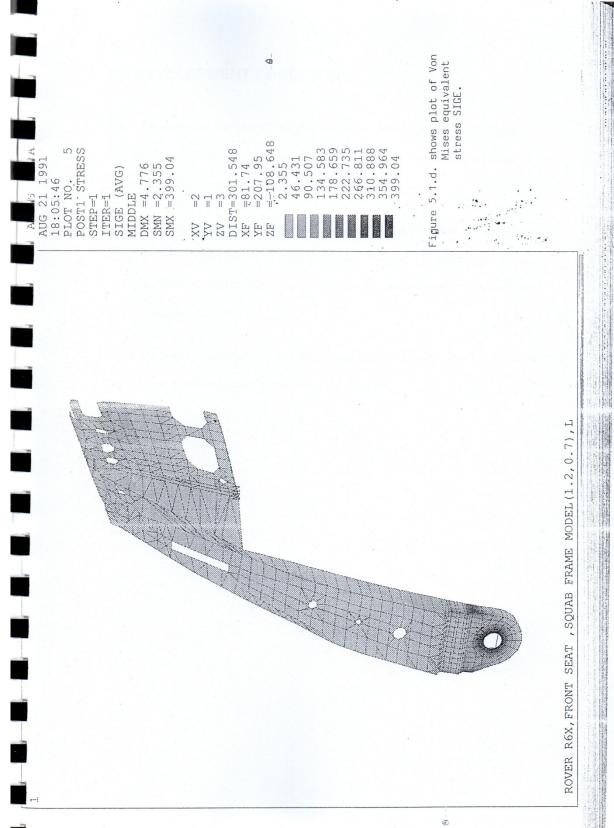
5.1 ORIGINAL CONDITION

Fig. 5.1.a. shows analysis results with the deflected shape in dark colour and the undeflected shape in the broken line. The plot shows clearly two views of the model the back face of the model being in tension while the front face is in compression. This model was subjected to a 51 kg object resulted in displacement of (4.776mm) at the top of the seat which would seem to be reasonable magnitude, however, this cannot be exact since experimental data were not available. Fig. 5.1.b. shows contour plot of the deflected shape, lowest and highest deflections in the model.

Fig. 5.1.c, Fig. 5.1.d, show plots of first principal and Von Mises equivalent stress. As it can be seen from these contour plots the maximum stresses concentrated around and near the lumber support bracketry.

Maximum values of SIG1 and SIGE are within the permissible stress level of mild steel (see Appendix H) in comparison to BS1449.



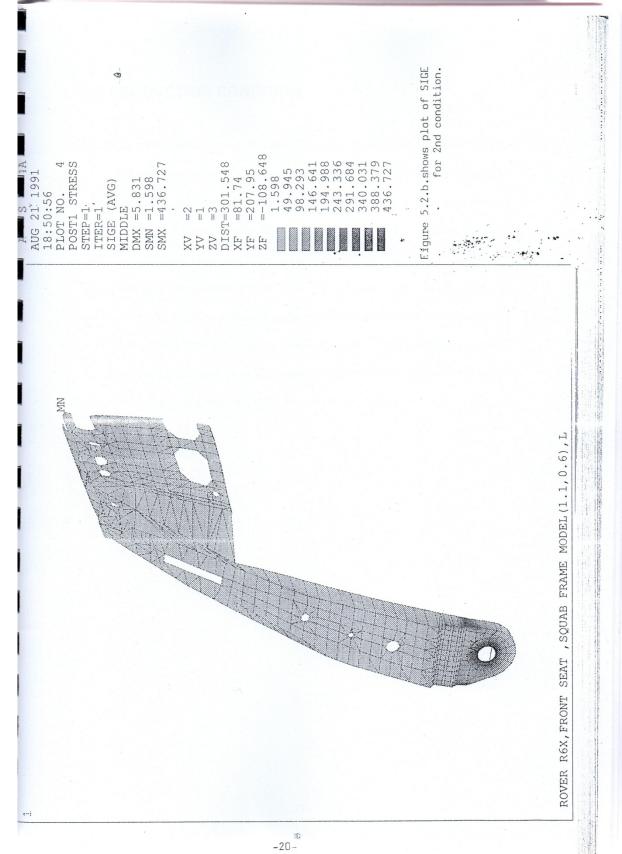


5.2 DEVELOPMENT CONDITION

Fig. 5.2.a. shows contour plot of first principal stress with value of (346.402 N/mm²). This plot is also showing the deflection value which is (5.831mm). The same amount of loading of original condition is applied to this condition.

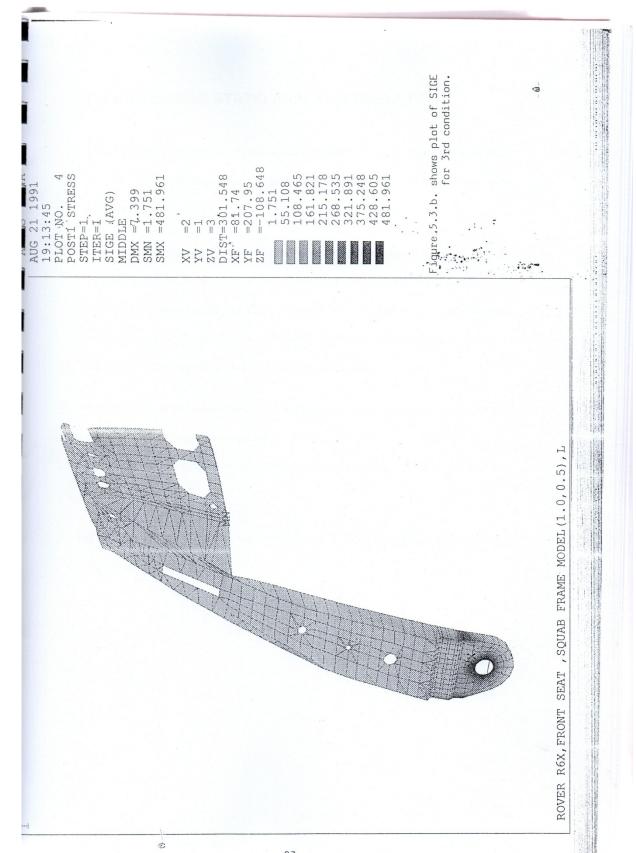
Since this model is smaller in thicknesses than the original condition, the deflection results is (1mm) higher than the original condition. Obviously this would cause a little bit higher level of stresses than the original condition, but it is still very near to the BS1449 values.

Fig. 5.2.b. shows contour plot of Von Mises equivalent stress with value of (436.727 N/mm²) and this value is valid in comparison to BS1449.



5.3 PRODUCTION CONDITION

Fig. 5.3.a. shows contour plot of first principal stress or SIG1 with maximum value of (396.779 N/mm²), on the same plot the deflection value of this model is (7.399mm). The loading is the same as with the other two previous conditions a 51kg in a form of unrestrained object in the rear seat of the car is thrown to forward direction. Since the thicknesses is smaller in this condition, therefore the deflection would be larger than the other conditions. Obviously this would cause a little bit higher level of stresses as it is the case in fig. 5.3.b, this plot shows Von Mises equivalent stress or SIGE with maximum value obtained (481.961 N/mm²) and to compare this value with that of BS value which is (480 N/mm²) it seems to be of acceptable result. However these results need to be compared with experimental data which were not available.



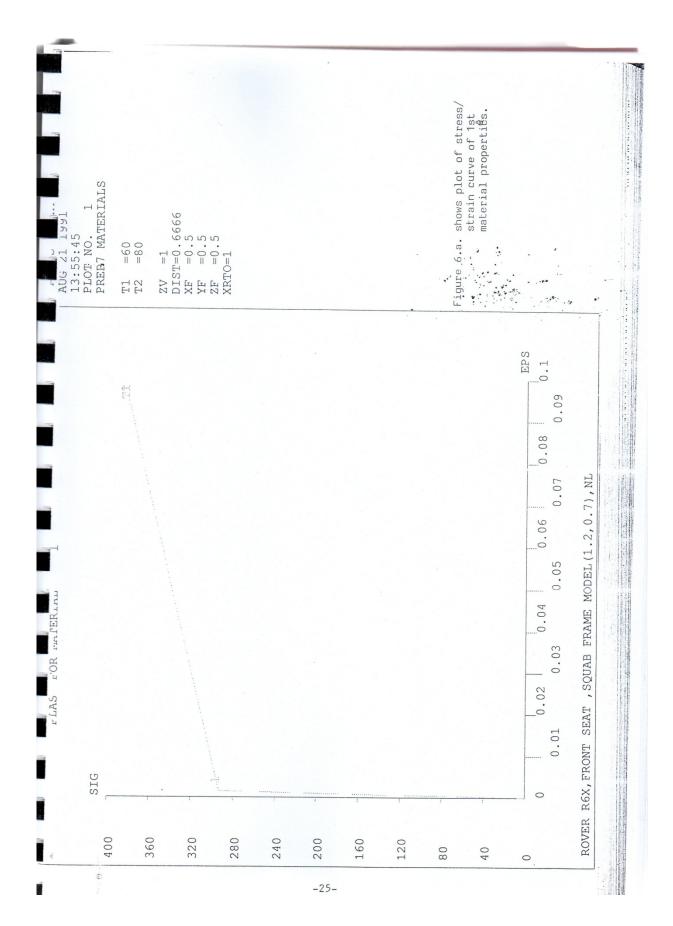
6.0 NON-LINEAR STATIC ANALYSIS RESULTS

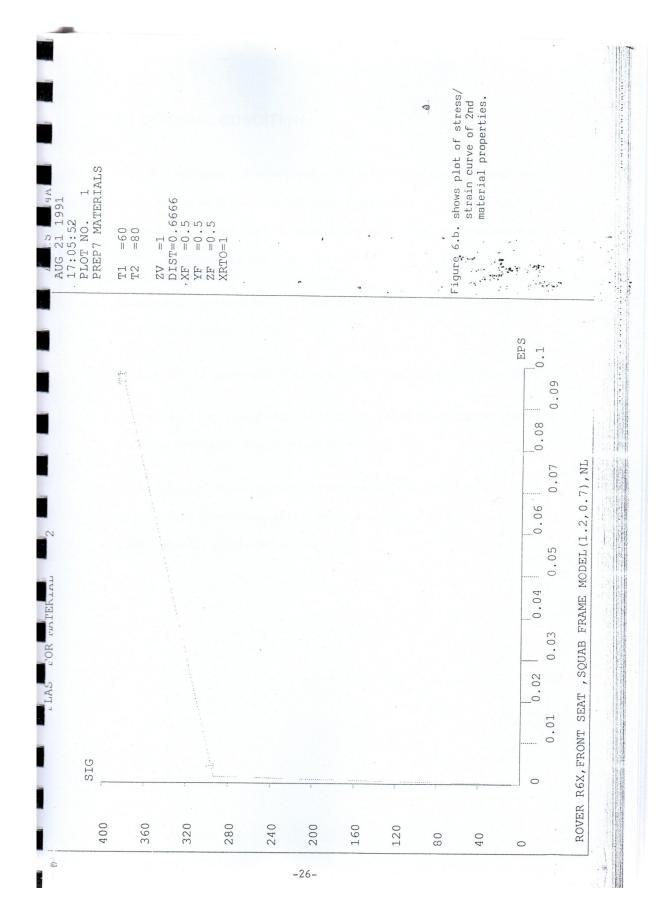
In this chapter only original condition was considered in which exactly has the same geometry, constrained and loading position as described earlier in the linear static analysis conditions.

Most of the engineering materials behave in linear pattern below stress level, and this called proportional limit. Below this limit, stress is in linear proportional to strain. Above the limit of proportionality and below the stress level the materials behave elastically and this is termed the yield point. Beyond the elastic limit the materials said to be in plastic region (i.e. any deformation can not be recoverable).

Incremental load was applied in a form of elements pressure. To check or to see whether this model or this analysis would behave in the same manner as described above, two plots of stress/strain curve of material properties for materials (1) and (2) are shown in Fig. 6.a, and Fig. 6.b.

The material which exhibits the same behaviour is called a bilinear material. See Appendices (C) and (D).





6.1 ORIGINAL CONDITION

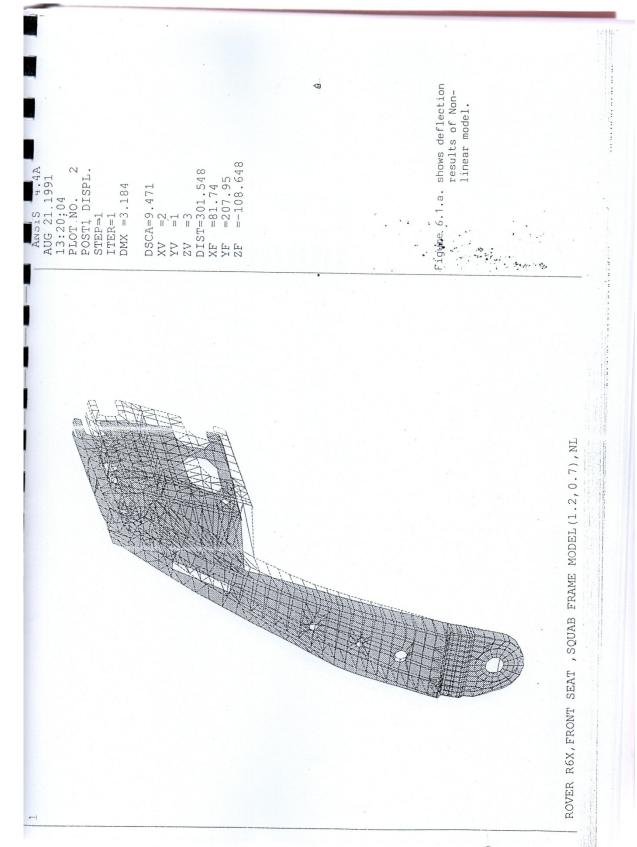
This model represents the non-linear static analysis. The loading used in this model was stepped loading. Fig. 6.1.a. shows deflection results for load step one and first iteration. Fig. 6.1.b. shows contour plot of deflected shape showing the maximum and minimum results obtained in the first step.

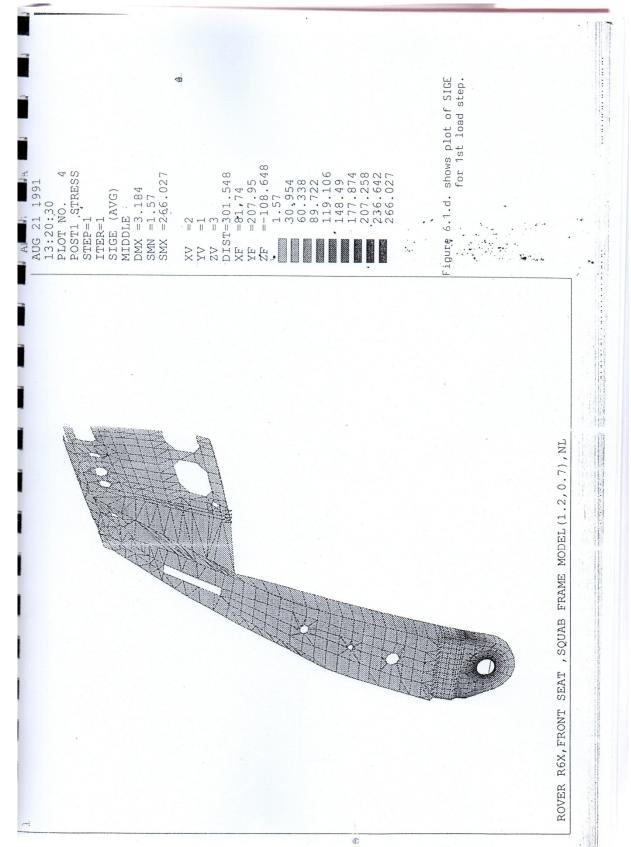
Fig. 6.1.c, fig. 6.1.d, are results of first principal stress (SIG1) and Von Mises equivalent stress (SIGE) of the first load step.

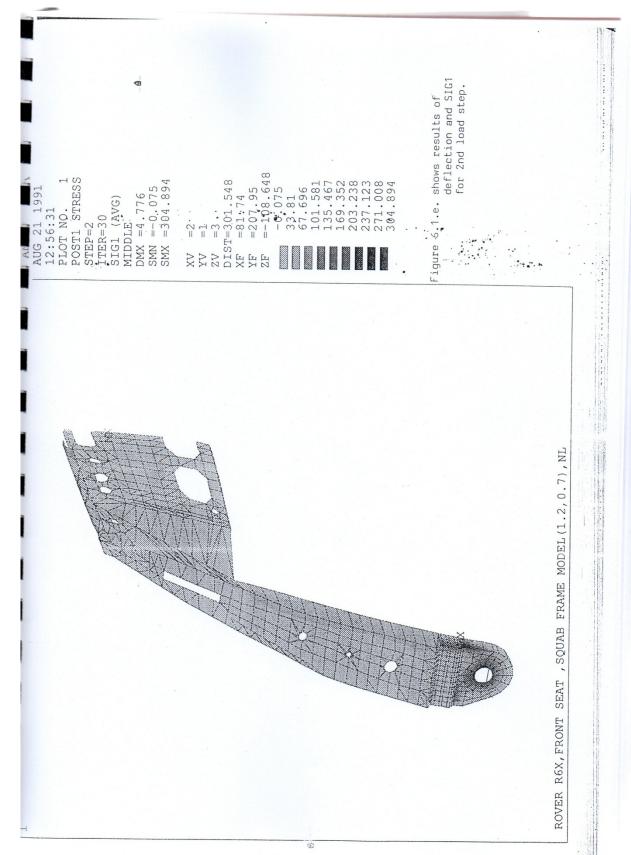
Fig. 6.1.e, fig. 6.1.f, shows results of second load step in maximum and minimum values.

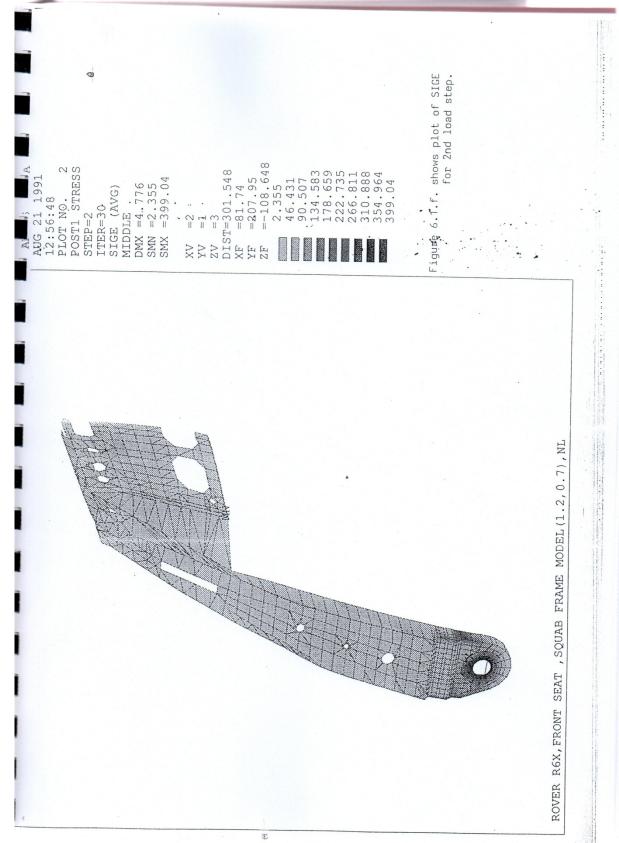
Fig. 6.1.g, fig. 6.1.h, represent third load step results of 68 kg object hitting the seat at 4 m/s^2 . SIGE is 10% higher than the permissible value.

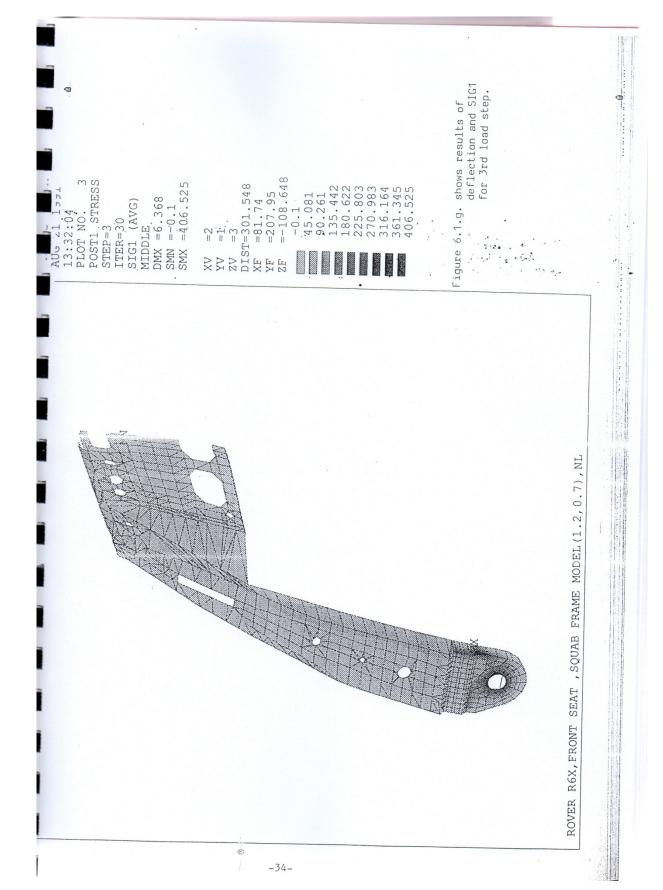
Fig. 6.1.i, fig. 6.1.j, shows the fourth load step results of 84 kg object at 4 m/s^2 this is the worst case that could happen as far as this investigation concerned. In this case SIGE is 38% higher than the BS 1449 value.

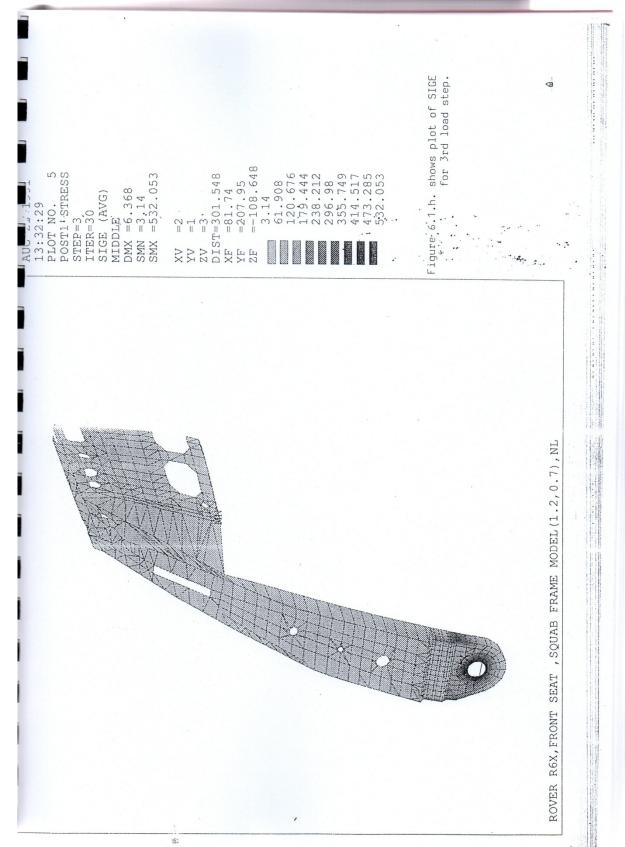


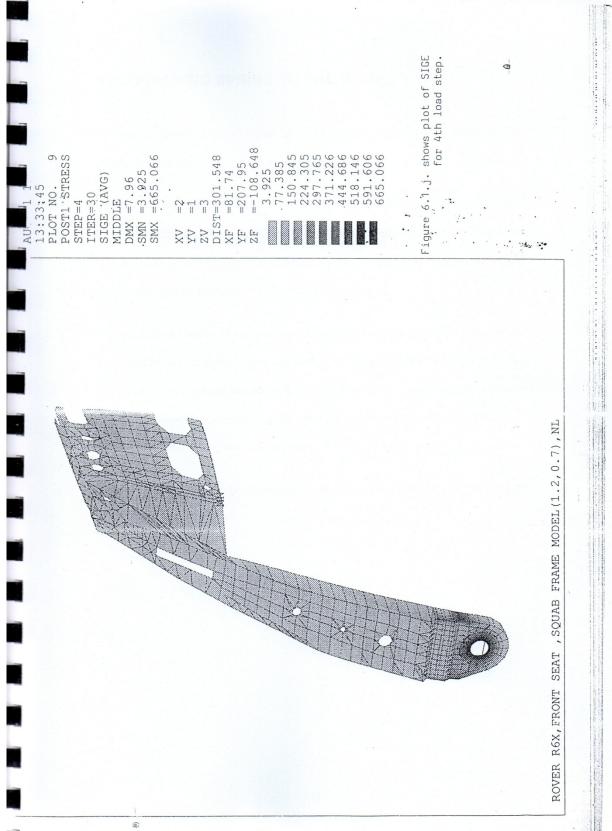












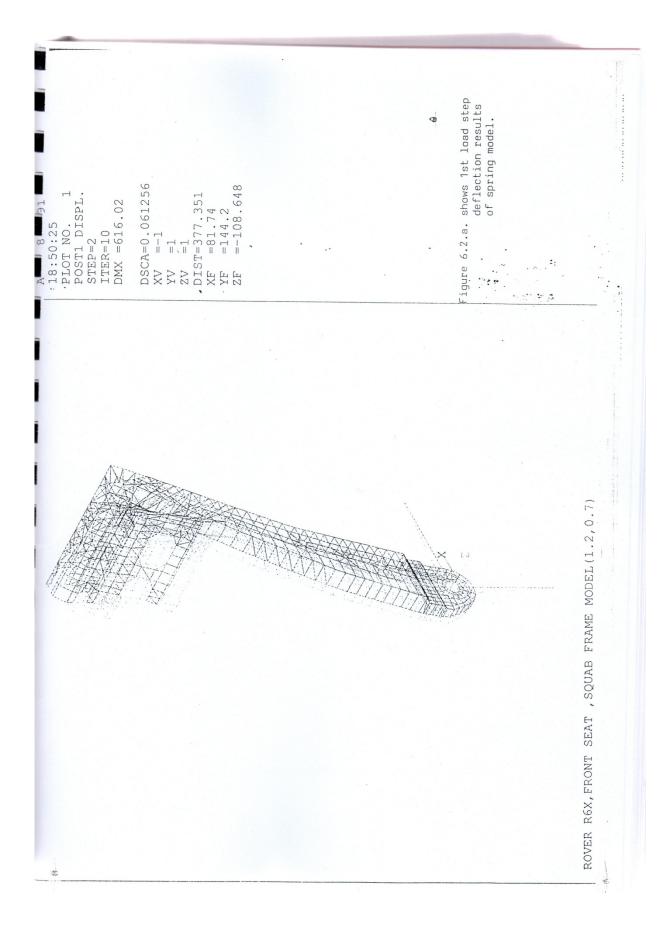
6.2 ATTACHING SPRING TO THE MODEL

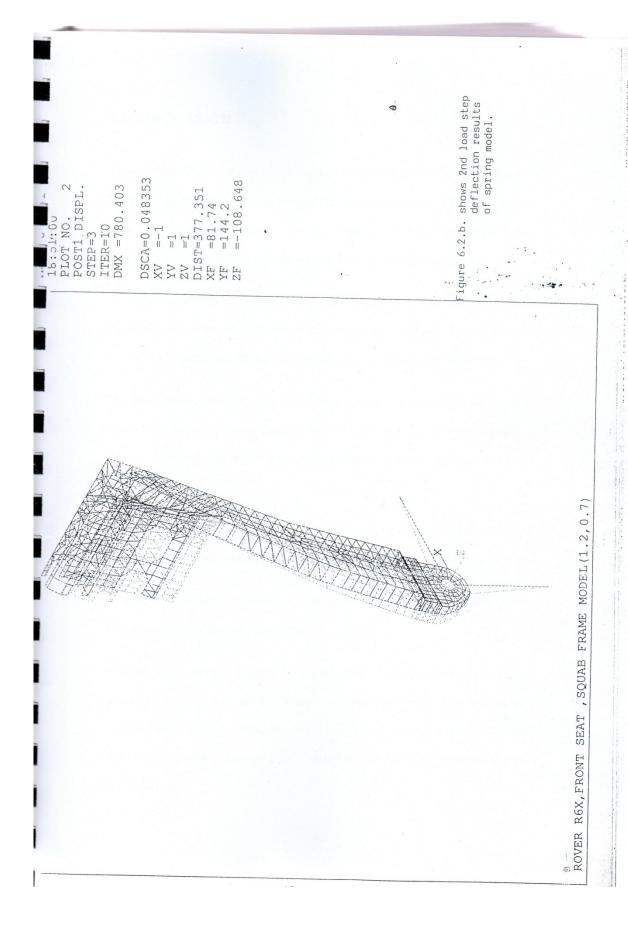
This model was an attempt of attaching spring from a fixed point to the seat at lumbar adjustment position.

The seat was constrained along the centre line in the Z-direction, and not constrained at lumbar adjustment position. The same type of loading was used as it was the case of Non-linear analysis data. STIF39 was used in addition to STIF63, STIF39 has one degree of freedom and two nodes and it is for Non-linear analysis.

Fig. 6.2.a. shows results of displacement shape with deflection more than half a metre, this indicates that the model with spring alone is not good enough to further improve the analysis without using any restraint at lumbar support bracketry or simply it is not rigid enough to stand the impact during sudden braking and even if the stiffness of the spring was increased the results would be the same.

Fig. 6.2.b. shows the results of the second step loading. See Appendix (C) of the data input of this model.





7.0 DISCUSSION

To make optimisation of cost and to improve weight efficiency three assumptions of modelling the seat have been made using Finite Element analysis. These assumptions were Linear static analysis for (original, development and production condition), and Non-linear static analysis for the original condition due to large displacement.

The Linear static analysis results of the above conditions were affected according to their thicknesses, i.e. in the thinnest material case (Production condition) the deflection was highest, and naturally the thickest material case the deflections were the lowest.

The first principal stresses in second and third assumptions were higher than the first assumption. And also the situation is the same with Von Mises equivalent stress (failure criteria). Obviously the smaller thickness would result in higher deflection and stress values.

The models geometry were extremely well constructed and consists of quadrilateral shell elements. The thicknesses for these conditions were reasonable for achieving the tests.

The deflection results appeared to be of acceptable magnitude and there were no big differences between the three conditions.

Similarly stress analysis obtained were reasonable. For original condition the first principal stress SIG1 was (304.894 N/mm²) and Von Mises equivalent stress SIGE was (399.04 N/mm²), and the deflection was (4.776 mm). For development condition the SIG1 value was (346.401 N/mm²), SIGE value was (436.727 N/mm²) and the deflection was (5.831mm).

For the last assumption of Linear static analysis SIG1 was (396.779 N/mm²), SIGE was (481.961 N/mm²) and the deflection was (7.399mm).

The results of SIG1, and SIGE of Linear static analysis for the three conditions indicate that there is no big difference between these conditions and in comparison with BS 1449 or permissible value. The displacements of the three conditions again were reasonable and only (1mm) difference between each of them.

The Non-linear static analysis results of the original condition was also good, and in this assumption only four steps of loading were used. In the first step SIG1 was (203.263 N/mm², SIGE was (266.027 N/mm²), and the deflection was (3.184mm). In the second step SIG1 was (304.894 N/mm², SIGE was (399.04 N/mm²) and the deflection was (4.776mm). In the third load step results of 68kg object hitting the seat at 4 m/s² were (406.525 N/mm² and (532.052 N/mm²) for SIG1 and SIGE respectively and the last value i.e. SIGE was 10% higher than the permissible value. The deflection was (6.368 mm).

In the fourth load step results of 84 kg object at 4 m/s 2 this load step was the worst case that could happen to the seat as far as this investigation was concerned. In this load step SIG1 was (508.156 N/mm 2) and SIGE was (665.066 N/mm 2) which is 38% higher than the permissible stress. The deflection for this load step was (7.96 mm).

The results obtained from the above analysis indicates that there were no big differences between the three assumptions in the Linear static analysis and in Non-linear analysis the results showed that there is a small percentage higher than the BS 1449 values, since 68 kg and 84 kg become huge masses when they are thrown with a certain acceleration

and from this argument therefore the cost can be optimised by choosing the concerned condition or masses in correlation with the experimental work.

8.0 CONCLUSIONS

The results of the three conditions in Linear static analysis were reasonable, first principal stress, Von Mises equivalent stress and deflection for original condition were (304.894 N/mm²), (399.04 N/mm²) and (4.776 mm) respectively and satisfy the BS1449-Part 1:83.

For development condition SIG1, SIGE were (346.402 N/mm²), (436.727 N/mm²) and deflection value was (5.831 mm).

Production condition values of SIG1, SIGE were (396.779 N/mm²), (481.961 N/mm²), and deflection value was (7.399mm).

The Non-Linear static analysis results were reasonably good with load step (1) values of SIG1, SIGE and the deflection were (203.263 N/mm²), (266.027 N/mm²) and (3.184 mm) respectively. Step (2) values were (304.894 N/mm²) for SIG1 and (399.04 N/mm²) for SIGE and (4.776 mm) for deflection. Step (3) values were (406.525 N/mm²) and (532.053 N/mm²) and (6.368 mm) for SIG1, SIGE and the deflection.

Results of last load step (4) were (508.156 N/mm^2) for SIG1 and (665.066 N/mm^2) and (7.96 mm) for SIGE and the deflection.

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- (3) ANSYS User get-started Manual. A.S. MANNING Nov. 86.
- (4) Swanson Analysis systems Ltd. (U\$A) ANSYS User Manual Revision 4.4 Vol. 1 and 2. ANSYS Command Reference Guide Revision 4.4
- (5) ANSYS Revision 4.2 Seminar Notes Structural Nonlinearities.
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- (7) AUSTIN ROVER

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 David S. Malkus
 Michael E. Plesha
 JOHN WILEY & SONS
- (2) BS1449: Part 1:1983
- (3) Mechanics of Materials 2nd Edition Vol. 1 and 2 E.J. HEARN PERGAMON PRESS

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The Principal Engineer of Technical Support, Body Trim

and Hardware.

APPENDIX C

EP, 900, 1, 0.15

EP, 901, 2, 0.15

ITER, 1, 1

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/TITLE, ROVER R6X, FRONT SEAT , SQUAB FRAME MODEL (1.2, 0.7), L
C*** ELEMENT TYPE1 IS STIF63 1.2MM THICK
ET, 1, 63
3,1,1.2
C*** ELEMENT TYPE2 IS STIF63 0.7MM THICK
ET, 2, 63
3,2,0.7
EX, 1, 207E3
NUXY, 1, 0.3
EX, 2, 207E3
VUXY, 2, 0.3
/COM, START OF THE ELEMENT MODULE
                             5,
                                      3
            4,
                     6,
E,
                              7,
3,
             6,
                     8,
    924, 172.4729, 297.1200, -100.4637
925, 167.8731, 288.3720, -103.0532
926, 165.6220, 286.8707, -102.1608
N.
N,
/COM, FINISH PREP7
/COM, THIS MODEL HAS AN ORIGINAL DESIGN THICKNESS OF:1.2MM FOR THE PANEL-
/COM, SIDE SQUAB AND OF 0.7MM FOR THE FRONT AND REAR PANEL-TOP-SQUABS, AND
/COM, PANEL-RAIL TRIM.
 ALBC, 1
KRF, 1
 D, 918, UZ, 0
0,915,UZ,,,916,1
 D, 912, UZ, 0
 D, 909, UZ, , , 910, 1
 0,736,UZ,0
 ),905,UZ,0
 D, 901, UZ, , , 902, 1
 D,898,UZ,0
 0,608,UZ,,,609,1
  D, 606, UZ, ,, 607, 1
 D, 665, UZ, , , 666, 1
  D, 663, UZ, , , 664, 1
  D, 244, ALL, , , 255, 1
  D, 258, ALL, , , 261, 1
  EP, 524, 1, 0.15, , 527, 1
  EP, 549, 1, 0.15
  EP, 751, 1, 0.15, , 752, 1
  EP,568,1,0.15,,570,1
  EP,737,1,0.15,,738,1
  EP, 572, 1, 0.15
   EP,590,1,0.15,,594,1
  EP, 611, 1, 0.15, , 615, 1
EP, 635, 1, 0.15, , 636, 1
  EP,648,1,0.15
   EP, 650, 1, 0.15
   EP, 898, 1, 0.15
   EP, 899, 2, 0.15
```

Figure C.1. shows input data of 3-D model, linear static analysis.

```
/TITLE, ROVER R6X, FRONT SEAT , SQUAB FRAME MODEL(1.2,0.7), NL
 C*** ELEMENT TYPE1 IS STIF63 1.2MM THICK
ET, 1, 63
R,1,1.2
C*** ELEMENT TYPE2 IS STIF63 0.7MM THICK
ET, 2, 63
R, 2, 0.7
EX, 1, 207E3
NUXY, 1, 0.3
EX, 2, 207E3
NUXY, 2, 0.3
CONV, 0.00001
TREF,70
TUNIF, 70
NL, 1, 13, 10
NL, 1, 25, 293, 293
NL, 1, 31, 0.94E3, 0.94E3
NL, 1, 19, 60, 80
NL, 2, 13, 10
NL, 2, 25, 293, 293
NL, 2, 31, 0.94E3, 0.94E3
NL, 2, 19, 60, 80
/COM, START OF THE ELEMENT MODULE
                  3,
E,
          4,
                  6,
                          5,
                                  3
     925,
N,
             167.8731,
                          288.3720, -103.0532
     926,
           165.6220,
                          286.8707, -102.1608
/COM, FINISH PREP7
/COM, THIS MODEL HAS AN ORIGINAL DESIGN THICKNESS OF:1.2MM FOR THE PANEL-
/COM, SIDE SQUAB AND OF 0.7MM FOR THE FRONT AND REAR PANEL-TOP-SQUABS, AND
ALBC, 1
KRF, 1
KBC, 0
D, 918, UZ, O
D, 915, UZ, , , 916, 1
D, 912, UZ, O
D,909,UZ,,,910,1
D, 736, UZ, 0
0,905,UZ,O
0,901,UZ,,,902,1
0,898,UZ,O
0,608,UZ,,,609,1
0,606,UZ,,,607,1
0,665,UZ,,,666,1
0,663,UZ,,,664,1
0,244,ALL,,,255,1
0,258,ALL,,,261,1
```

Figure C.2. shows input data of 3-D model, Non-linear static analysis.

```
/TITLE, ROVER R6X, FRONT SEAT , SQUAB FRAME MODEL(1.2,0.7)
KNL, 1
C*** ELEMENT TYPE1 IS STIF63 1.2MM THICK
ET, 1, 63
R,1,1.2
C*** ELEMENT TYPE2 IS STIF63 0.7MM THICK
ET, 2, 63
R, 2, 0.7
EX, 1, 207E3
NUXY, 1, 0.3
EX, 2, 207E3
NUXY, 2, 0.3
C*** ELEMENT TYPE3 AND TYPE3 ARE STIF39
ET, 3, 39,,,1
C*** ELEMENT TYPE4 AND TYPE4 ARE STIF39
ET, 4, 39,,,2
KAY, 3, 5
KAY, 6, 1
CONV, 0.0001
TREF, 70
TUNIF, 70
NL, 1, 13, 10
NL, 1, 25, 293, 293
NL, 1, 31, 0.94E3, 0.94E3
NL, 1, 19, 60, 80
NL, 2, 13, 10
NL, 2, 25, 293, 293
NL, 2, 31, 0.94E3, 0.94E3
NL, 2, 19, 60, 80
/COM, START OF THE ELEMENT MODULE
E,
                    3,
           4,
                            1,
                                    2
E,
           4,
                  6,
                            5,
                                    3
                          288.3720, -103.0532
286.8707, -102.1608
      925, 167.8731,
N,
N,
      926,
              165.6220,
N,
      927,
              160.0000,
                          -5.94377E-4 , -13.800
N,
/COM, FINISH PREP7
     928,
             0
                             -160.00 , -13.800
/COM, THIS MODEL HAS AN ORIGINAL DESIGN THICKNESS OF: 1.2MM FOR THE PANEL-
/COM, SIDE SQUAB AND OF 0.7MM FOR THE FRONT AND REAR PANEL-TOP-SQUABS, AND /COM, PANEL-RAIL TRIM.
ALBC, 1
KRF, 1
KBC, 0
D, 918, UZ, O
D, 915, UZ, , , 916, 1
D, 912, UZ, 0
D, 909, UZ, , , 910, 1
D,736,UZ,0
D, 905, UZ, 0
D, 901, UZ, , , 902, 1
D,898,UZ,0
D,608,UZ,,,609,1
D, 606, UZ, , , 607, 1
D, 665, UZ, , , 666, 1
D, 663, UZ, , , 664, 1
TYPE, 3
REAL, 3
E, 927, 255
E, 928, 251
R, 3, 0.0, 0.0, 0.1, 2E5, 0.2, 2E6
R, 4, 0.0, 0.0, 0.1, 2E5, 0.2, 2E6
D, 927, ALL
D, 928, ALL
                                          Figure C.3. shows input data of 3-D model,
```

Non-linear, spring model.

```
TER, 1, 1, 1
 2,524,1,0.1,,527,1
2,549,1,0.1
   P,751,1,0.1,,752,1
 2P, 568, 1, 0.1, ,570, 1

2P, 737, 1, 0.1, ,738, 1

2P, 572, 1, 0.1

2P, 590, 1, 0.1, ,594, 1

3P, 611, 1, 0.1, ,615, 1
   P, 635, 1, 0.1, , 636, 1
   P,648,1,0.1
 P, 650, 1, 0.1
P, 898, 1, 0.1
   P,899,2,0.1
  P,900,1,0.1
P,901,2,0.1
 LWRITE
TER, -30,,30
P,524,1,0.15,,527,1
EP,549,1,0.15
EP,751,1,0.15,,752,1
P,568,1,0.15,,570,1
EP,737,1,0.15,,738,1
EP,572,1,0.15
   P,590,1,0.15,,594,1
 P,611,1,0.15,,615,1
P,635,1,0.15,,636,1
EP,648,1,0.15
P,650,1,0.15
  P,898,1,0.15
 EP, 899, 2, 0.15
  P, 900, 1, 0.15
   P, 901, 2, 0.15
   WRITE
```

03

Figure C.4. shows the data of load step 1 and 2.

```
[TER, -30,,30
EP, 524, 1, 0.2, , 527, 1
EP, 549, 1, 0.2

3P, 751, 1, 0.2, , 752, 1

3P, 568, 1, 0.2, , 570, 1

EP, 737, 1, 0.2, , 738, 1
EP, 572,1,0.2

EP, 572,1,0.2

EP, 590,1,0.2,,594,1

EP, 611,1,0.2,,615,1

EP, 635,1,0.2,,636,1

EP, 648,I,0.2
EP, 650, 1, 0.2
EP, 898, 1, 0.2
 EP,899,2,0.2
 EP, 900, 1, 0.2
 EP, 901, 2, 0.2
LWRITE
 ITER, -30,,30
 EP,524,1,0.25,,527,1
EP, 549,1,0.25
EP, 751,1,0.25,,752,1
EP, 568,1,0.25,,570,1
EP, 737,1,0.25,,738,1
EP, 572,1,0.25
EP,590,1,0.25,,594,1
EP,611,1,0.25,,615,1
EP,635,1,0.25,,636,1
EP,648,1,0.25
 EP,650,1,0.25
 EP,898,1,0.25
  EP,899,2,0.25
 EP, 900, 1, 0.25
  EP, 901, 2, 0.25
```

LWRITE

Figure C.5. shows the data of load step 3 and 4.

APPENDIX D (LOAD CALCULATIONS)

1.0 Calculations of the area of pressure load

Length = 149 mm

Width = 11.125 mm

Number of elements = 31

Area =
$$A_1 = L_1 * \omega_1 = 149*11.125 = 1657.625 \ mm^2$$

Total Area = $A_1 * 2 = 1657.625 * 2 = 3315.25 mm^2$

Linear static analysis

It has been assumed for this analysis that 0.15 N of pressure load on each element.

Force = Pressure * Area

F = 0.15 * 3315.25 = 497 N

and $mass = \frac{Force}{acceleration} = \frac{497}{9.81} = 51 \text{ kg mass of the object at rest}$

Car at speed of 25 mph

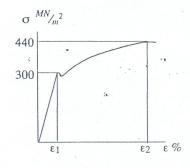
 $\frac{25 * 1.61 * 10^3}{3600} = 11.18 \text{ m/s} \text{ when the car takes a sudden brake at time } t = 3 \text{ Sec}$

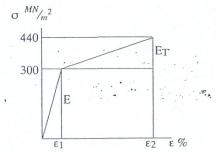
$$a = \frac{v}{t} = \frac{11.18}{3} = 3.73 \, \frac{m}{s^2}$$

F = m a

and $m = \frac{F}{a} = \frac{497}{3.73} = 133.2 \text{ kg}$ the mass of the object during impact.

Non-linear static analysis





 $E = Modulus of elasticity = 207 * 10^3 N/mm^2$

 E_T = The plastic slope

$$E = \frac{stress}{strain} = \frac{\sigma}{\varepsilon}$$

$$\varepsilon_1 = \frac{\sigma}{E} = \frac{300}{207 * 10^3} = 1.449 * 10^{-3} \text{ or } 0.1449 \%$$

 $\epsilon_2 = 0.15$ (for mild steel)

$$\Delta \varepsilon = \varepsilon_2 - \varepsilon_1 = 0.15 - 1.449 * 10^{-3} = 0.148551 \approx 0.149$$

$$\Delta \sigma = \sigma_2 - \sigma_1 = 440 - 300 = 140 \frac{N}{nm^2}$$

$$E_T = \frac{\Delta \sigma}{\Delta \epsilon} = \frac{140}{0.149} = 940 \frac{N}{mm^2}$$

Plasticity ratio =
$$\frac{E_T}{E} = \frac{940}{207 * 10^3} = 4.54 * 10^{-3} \approx 0.00454$$

when this ratio is less than 0.05 (5%) use 0.05

. Load increments no larger than 0.05

Hence the load increments would be as follows:-

Load Steps (N)	Mass at rest (kg)	Mass at impact (kg)
0.1	34	88.4
0.15	51	133.2
0.2	68	178.0
0.25	84	222.2

Verifications

Calculation of deflections

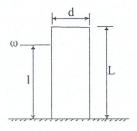
$$Y = \frac{\omega L^2}{2EI} (L - \frac{l}{3})$$

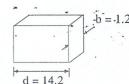
$$\omega = 0.15 * 31 = 4.65 N$$

$$L = 448.4 \text{ mm}$$

$$E = 207 * 10^3 \text{ N/mm}^2$$

$$1 = 399 \text{ mm}$$





$$I = \frac{bd^3}{12}$$

$$Y_o = \frac{4.65 * (448.4)^2}{2 * 207 * 10^3 * 286.3} (448.4 - \frac{399}{3})$$

$$Y_o = 2.6 \text{ mm (deflection of original condition)}$$

$$I = \frac{1.2(14.2)^3}{12} = 286.3 \, mm^4$$

$$I = \frac{1.1 (14.2)^3}{12} = 262.5 mm^4$$

 Y_d = 2.9 mm (deflection of development condition)

$$I = \frac{1.0 (14.2)^3}{12} = 238.6 mm^4$$

 $Y_p = 3.2 \text{ mm}$ (deflection of production condition)

In the same way the deflections of non-linear analysis would be calculated.

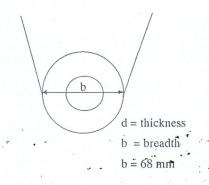
Calculation of stresses

$$\sigma = \frac{MY}{I}$$

$$Y_o = \frac{1}{2}t = \frac{1}{2} * 1.2 = 0.6 mm$$

$$\dot{Y}_d = \frac{1}{2} * 1.1 = 0.55 \ mm$$

$$Y_p = \frac{1}{2} * 1.0 = 0.5 mm$$



$$M = 4.65 * 149 = 692.85 N mm$$

$$\sigma_o = \frac{692.85 * 0.6}{9.792} = 42.45 \frac{N}{mm^2}$$

$$\sigma_d = \frac{692.85 * 0.55}{7.542} = 51 \frac{N_{mm}^2}{}$$

$$\sigma_p = \frac{692.85 * 0.5}{5.67} = 61 \frac{N_{mm}^2}{}$$

$$I = \frac{bd^3}{12}$$

$$I = \frac{68(1.2)^3}{12} = 9.792 \, mm^4$$

$$I = \frac{68(1.1)^3}{12} = 7.542 \, mm^4$$

$$I = \frac{68(1.0)^3}{12} = 5.67 \, mm^4$$

```
IPPENDIX E.
              0.
                        LOAD CASE= 1
THE FOLLOWING X, Y, Z DISPLACEMENTS ARE IN GLOBAL COORDINATES
            UX
                             UY
  767 -2.6689364
                                                              ROTX
                                                                               ROTY
                          1.5084818
                                      0.91218720E-02 -0.32103880E-02 -0.95958868E-02:
  770
       -2.5754233
                                       0.11567196E-01 -0.46514627E-02 -0.11930036E-01:
0.14048104E-01 -0.11621479E-02 -0.12143834E-01
                         1.4822371
  771
       -2.6378747
                         1.5029220
  773
       -2.5835662
                          1.4747690
                                      -0.10770015E-01 0.60396702
  774
                                                                       -0.91396523
       -2.4987540
                                       0.11519328E-01 -0.37683230E-02 -0.12606213E-01
0.17694837E-01 -0.37167556E-02 -0.10574147E-01
                         1.4582022
  775
       -2.5722124
                         1.4850247
  776
       -2.5050643
                                       -0.10722309E-01 -0.69309795E-01 0.83728965E-01 -0.23856936E-01 0.91588486E-03 0.33693474E-02 0.26883121E-01 0.11902302E-02 0.35468595E-02
                          1.4481273
  777
       -3.5489066
                          1.7392126
  779. -3.5283335.
780 -3.5123035
                          1.7319429
                    1.7296868
                                      -0.31532303E-01 -0.18729279E-02 0.47804772E-02
  781
       -3.5171315
                     1.7322346
                                      -0.35116745E-01 0.20869602E-02 0.59568176E-02 -0.34598510E-01 0.18763102E-02 0.64581402E-02
  782
       -3.5487803
                         1.7395917
        ***** POST1 NODAL DISPLACEMENT LISTING *****
LOAD STEP
         0.
             1 ITERATION= 1 SECTION= 1
                      LOAD CASE= 1
EE FOLLOWING X, Y, Z DISPLACEMENTS ARE IN GLOBAL COORDINATES
 NODE
           UX
                            IIY
 783 -3.5872134
                                             UZ
                                                             ROTX
                        1.7496002
                                                                              ROTY
                                      -0.26700506E-01 0.92241400E-03 0.52188700E-02
 784
      -3.5685968
                                      1.7463228
 785
      -3.5819348
                        1.7463753
1.7431760
 786
      -3.5594663
 787
      -3.4638997
                                      -0.35853571E-01 0.90674198E-02 0.31324671E-02
-0.34871040E-01 0.20308339E-02 0.44516281E-02
                        1.7203871
 788
      -3.5226403
                        1.7276604
 789
      -3.1363756
                        1.6252020
                                      -0.16781793E-01 0.51679521E-03 -0.44497002E-03
 790
      -3.2866778
                        1.6889114
                                      -0.31817041E-01 0.19373953E-02 0.14926740E-02
 791
      -3.1308123
                                       -0.15363500E-01 -0.30194649E-02 0.45932748E-03
                        1.6109541
 793
      -2.7639730
                                       0.17175009E-01 0.28227010E-03 -0.71089666E-02
                        1.5403615
794
      -2.9863058
                        1.5785094
                                       -0.32848840E-02 -0.28673688E-02 -0.25222605E-02
795
      -2.7009420
                        1.5272240
                                       0.23263850E-01 0.34314788E-02 -0.41509875E-02:
      ***** POST1 NODAL DISPLACEMENT LISTING *****
AD STEP
           1 ITERATION= 1 SECTION= 1
WE=
           0. LOAD CASE= 1
FOLLOWING X, Y, Z DISPLACEMENTS ARE IN GLOBAL COORDINATES
          UX
                           UY
                                            UZ.
796
                                                            ROTX
     -2.9463690
                        1.5546350
                                      -0.21174028E-02 -0.29420796E-02 -0.27046116E-02
797
      -2.5607631
                                      0.21601501E-01 -0.13222194E-02 -0.92497662E-02
                        1.4839299
798
     -2.6816519
                                      0.20889129E-01 -0.72228342E-05 -0.85563933E-02
                        1.5187176
799
     -2.5971838
                       1.5086086
                                      0.34330343E-01 -0.30742559E-03 -0.63639493E-02
800
                                   0.27619342E-01 0.27922395E-03 -0.10605578E-01
     -2.4578830
                      1.4572650
801
     -2.5254199
                        1.4736403
                                   0.22377938E-01 -0.64579039E-02 -0.83181105E-02
802
     -2.5332523
                        1.4777776
                                       0.24328935E-01 -0.84274190E-02 -0.83098128E-02
803
     -2.4369943
                       1.4484527
                                      0.25132604E-01 -0.48124576E-02 -0.10918469E-01
804
     -2.4290130
                        1.4438169
                                      0.23032806E-01 -0.81083705E-02 -0.10668410E-01
807
     -2.3411420
                                   0.11321437E-01 -0.45088548E-02 -0.13038761E-01
                      1.4069514
BO 8
     -2.3395557
                                     -0.11572389E-01 0.79967903E-03 -0.21559398E-01
                       1.3910000
E12
     -3.6752567
                       1.7815274
                                     -0.16868208E-01 0.14445605E-02 0.50965715E-02
```

***** POST1 NODAL DISPLACEMENT LISTING *****

STEP 1 ITERATION= 1 SECTION= 1 E= 0. LOAD CASE= 1

FOLLOWING X, Y, Z DISPLACEMENTS ARE IN GLOBAL COORDINATES

```
242 104.75354
243 174
                                           174.83178
                                                                                                152.10450
      244
             236.60408
                                                                                                288.48929
                                                                         449.14654
433.19260
      245
             221.44474
                                                    -211.74786
                                            0.
                                                                                                394.89926
                                                                                                380.65642
             ***** POST1 NODAL STRESS LISTING ****
   LOAD STEP 1 ITERATION= 1 SECTION= 1
TIME= 0. LOAD CASE= 1
  SHELL STRESSES ARE AT MIDDLE
                                NODE
                  STG1
   246 207.07356
247 210.35160
248 219.14983
249 214.15198
250 196.54375
          179.83800
    251
    252
           174.03994
185.32838
                                                                          361.60613
391.55672
311.84507
317.91082
    253
    254
           213.63550
           241.65633
161.04126
    255
    257
    258
           241.67981
222.31026
    259
           ***** POST1 NODAL STRESS LISTING *****
 LOAD STEP 1 ITERATION= 1 SECTION= 1 TIME= 0. LOAD CASE= 1
                        LOAD CASE= 1
SHELL STRESSES ARE AT MIDDLE
          SIG3 SI SIGE

232.66563 0. -215.74730 435.07002 382.34066

177.43649 0. -221.36135 454.02698 399.03982 /

62.857970 -0.25527437E-09 -21.664953 84.522924 76.878968

44.770351 0.71724875E-10 -30.073693 74.844044 66.641565

23.079848 0.18553109E-09 -12.727545 35.807393 31.963225

12.802031 0.60767449E-10 -13.834861 26.636892

11.386179 0.47042083E-10 -21.580224
  260 219.32272
261 232.66563
262 177.43649
   263
   263
264
   265 44.770351
        23.079848
   266
   267
                                                                34.237136
26.636892
29.847682
28.867073
24.943993
30.226570
  268
   269
  270
                                     0. -17.480894
0. -10.844094
  271
        14.099899
18.782196
                                                                                            25.206816
                                                                                            21.698622
                                        0.
                                                 -11.444374
         ***** POST1 NODAL STRESS LISTING ****
LOAD STEP 1 ITERATION= 1 SECTION= 1
TIME= 0. LOAD CASE= 1
ELL STRESSES ARE AT MIDDLE
               SIG1
                                    SIG2
273 18.507201
274 29.289722
275 22.336613
                                                      SIG3
                                                                24.625818
34.456319
31.365544
                                  0. -6.1186165
0. -5.1665973
0. -9.0289315
                                                                            SI
                                                                                                 SIGE
                                                 -6.1186165
                                                                                            22.239534
                                                                                            32.200016
                                                                 31.365544
27.974419
25.743860
34.493602
        24.982018
20.428290
                                     0.
 276
                                                                                            27.968426
                                                -2.9924010
                                                                                           26.610350
        278
                                                                                        23.543375
 279
                                                                                           33.160619
 281
                                                                   27.279151
                                                                                        26.531749
9.3389519
                                                                 27.279151
10.198342
15.135724
9.6738544
13.221318
 283
        12.272951
0.25573355
                           -1.2409914 -9.4181208
-0.64460871 -12.955027
                                                                                           13.973940
 285
       0.26629175
                                                                                        9.1205596
                        -0.64460871
      0.25077695 -0.88568556E-01 -37.410481
                                                                                          12.794351
                                                                     37.661258
                                                                                         37.492897
```

```
SIG2 SIG3 SI SIGE

0. -377.39669 722.51930 635.03772

0. -390.07738 740.66338 651.27501

-364.14329 729.39301 641.18213

678.12501 596.53865

545.94357
                                                            SIG2
 NODE
                            SIG1
  246 345.12261
                    350.58599
365.24972
      247
                                                                    0. -364.14329 729.39301 596.53865

0. -321.20504 678.12501 596.53865

0. -293.18121 620.75412 545.94357

0. -291.63709 591.36709 519.74178

0. -312.61032 602.67689 529.85136

0. -343.71390 652.59453 574.11677

0. -359.84004 715.89921 629.52711

0. -338.59334 741.35390 652.37841

0. -214.44739 482.84949 423.79809

0. -311.65260 714.45229 629.56080
     248
     249
                  356.91997
      250
                     327.57291
                    299.73001
      251
                    290.06657
     252
                  308.88063
     253
      254
                  356.05917
                    402.76055
      255
      257
                   268.40210
                                                                        0. -311.65260
0. -324.93400
     258 402.79969
259 370.51710
                                                                                                                                                             611.39634
                                                                                                                        695.45110
**** POST1 NODAL STRESS LISTING *****
 LOAD STEP 4 ITERATION= 30 SECTION= 1
TIME= 0. LOAD CASE= 1
                                                                                                                                    SHELL STRESSES ARE AT MIDDLE
                                                               SIG2 SIG3 SI SIGE

0. -359.57883 725.11670 637.23444

0. -368.93558 756.71163 665.06637

0. -261.99371 557.72119 486.57374

2547682E-09 -36.108256 140.87154 128.13161

0. -113.62272 269.30619 236.42805

1952970E-09 -50.122821 124.74007 111.06927

9039965E-09 -21.212574 59.678988 53.272041

9039965E-09 -26.536811 57.061894 50.075875

0128959E-09 -23.058101 44.394819 38.482975

8400983E-10 -35.967039 49.746137 44.534912

0. -29.134823 48.111788 42.011359

0. -18.073490 41.573321 36.164370

0. -19.073957 50.377617 44.058987
                             SIG1
  NODE
                    365.53787
     260
      261/
                   387.77605
                 262
                     295.72748
      263
      264
      265
      266
       267
       268
       269
       270
       271
       272
```

APPENDIX G TABLES OF RESULTS

Model	Dmx displacement (mm)	SIG1 N/mm ²	SIG2 N/mm ²	SIG3 N/mm ²	SIGE N/mm ²
Original Condition	4.776	maximum	maximum	maximum	maximum
D40r6x (1.2,0.7)		304.894	52.627	0.768E-07	399.04
		minimum	minimum	minimum	minimum
		-0.075	-54.833	-331.003	2.355
Development condition	5.831	maximum	maximum	maximum	maximum
041r6x 1.1,0.6)		346.402	59.636	0.661E-07	436.727
		minimum	minimum	minimum	minimum
		-0.075	-59.395	-374.437	1.598
ondition	7.399	naximum	maximum	maximum	maximum
42r6x .0,0.5)	3	96.779	68.043	0.582E-07	481.961
	n	ninimum	minimum	minimum	minimum
	-(0.075	-64.557	-426.845	1.751

Table (1) results of Linear Analysis (full load).

Model	Dmx displacement (mm)	SIG1 N/mm ²	SIG2 N/mm ²	SIG3 N/mm ²	SIGE N/mm ²
Original	3.582	maximum	maximum	maximum	maximum
Condition D40r6x		228.7	39.5	0.576E-07	299.3
(1.2,0.7)		minimum	minimum	minimum	minimum.
		-0.06	-41.1	-248.3	1.766
Development	4.373	maximum	maximum	maximum	maximum
Condition D41r6x		260.0	44.73	0.661E-07	327.5
(1.1,0.6)		minimum	minimum	minimum	minimum
		-0.06	-44.55	-281.0	1.198
Production	5.549	maximum	maximum	maximum	maximum
Condition D42r6x		297.6	51.0	0.582E-07	361.5
(1.0,0.5)		minimum	minimum	minimum	minimum
		-0.06	-48.42	-320.0	1.31

Table (2) results of Linear Analysis (3/4 load).

Model	Dmx displacement (mm)	SIG1 N/mm ²	SIG2 N/mm ²	SIG3 N/mm ²	SIGE N/mm ²
Original Condition	2.388	maximum	maximum	maximum	maximum
D40r6x (1.2,0.7)		152.45	26.314	0.384E-07	199.52
;		minimum	minimum	minimum	minimum
		-0.0375	-27.416	-165.5	1.177
Development Condition		maximum	maximum	maximum	maximum
D41r6x (1.1,0.6)		173.2	29.818	0.331E-07	218.4
		minimum	minimum	minimum	minimum
		-0.0375	-29.697	-187.2	0.799
Production Condition	3.699	maximum	maximum	maximum	maximum
D42r6x (1.0,0.5)		198.39	34.0	0.291E-07	241.0
,,		minimum	minimum	minimum	minimum
		-0.0375	-32.278	-213.42	-0.876

Table (3) results of Linear Analysis (1/2 load).

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Model	Dmx displacement (mm)	SIG1 N/mm ²	SIG2 N/mm ²	SIG3 N/mm ²	SIGE N/mm ²
D27r6x	3.184	maximum	maximum	maximum	maximum
(1.2,0.7) set,1,1		203.263	35.085	0.512E-07	266.027
		minimum	minimum	minimum.	minimum
		-0.05	-36.085	-220.669	1.57
set,2,30	4.776	maximum	maximum	maximum	maximum
		304.894	52.627	0.768E-07	399.04
		minimum	minimum	minimum	minimum
		-0.075	-54.833	-331.003	2.355
set,3,30	6.368	maximum	maximum	maximum	maximum
		406.525	70.17	0.102E-06	532.053
		minimum	minimum	minimum	minimum
		-0.1	-73.111	-441.337	3.14
set,4,30	7.96	maximum	maximum	maximum	maximum
		508.156	87.712	0.128E-06	665.066
		minimum	minimum	minimum	minimum
		-0.125	-91.389	-551.672	3.925

Table (4) results of Non-linear Analysis

a.5.2 Weldability. All the grades specified in tables 10 and 12 shall be weldable provided that the welding techniques employed make allowance for composition and thickness. See BS 693, BS 1140, BS 2630, BS 5135 and BS 6265.

3.5.3 Strain-age-embrittlement. Where proof of freedom from strain-age-embrittlement is required (see 3.2(g)), the method of test shall be agreed between the manufacturer and the purchaser, as the test defined in 1.11 may not be appropriate to all the steels in this section.

Table 13. Mechanical properties: micro-alloyed steels

Grade	Rolled	Yield strength,	Tensile strength.	Elongat	ion, A, min.		Bend mandre
(see note 1)		R _e , min.	R _m , min.	Original	(180° bend)		
				50 mm	80 mm (note 4)	200 mm	(see note 3)
		N/mm²	N/mm ²	%	%	%	
40/30	HR, HS, CS	300	400	26	(24)	18	2a
43/35	HR, HS, CS	350	430	23	(21)	16	28
46/40	HR, HS, CS	400	460	20	(18)	12	3a
50/45	HR, HS, CS	450	500	20	(18)	12	3a
60/55	-, HS, CS	550	600	17	(15)	10	3.5a
40F30	HR, HS, CS	300	400	28	(26)	20	0a
43F35	HR, HS, CS	350	430	25	(23)	18	0.5a
46F40	HR, HS, CS	400	460	22	(20)	14	1 <i>a</i>
50F45	HR, HS, CS	450	500	22	(20)	14	1.5a
60F55	-, HS, CS	550	600	19	(17)	11	1.5a
68F62	-, HS -	620	680	18	(16)	10	2 <i>a</i>
75F70	-, HS -	700	750	15	(13)	8	3 <i>a</i>

a is the thickness of the bend test piece.

NOTE 1. The properties of HS materials are only applicable up to and including 8 mm. For material thicker than 8 mm, the properties are to be agreed between the manufacturer and purchaser.

NOTE 2. A specific range for the yield strength of any particular grade and thickness may be agreed between manufacturer and purchaser at the time of ordering.

NOTE 3. The bend test requirements quoted in this table are for specially prepared test pieces (see 1.10): conditions during fabrication may be more severe and not simulate those during laboratory testing (see note, 'Manipulation', to section three and table 14).

NOTE 4. The 80 mm gauge length is currently not used in the UK but, as a step towards conforming with European practice, tentative values have been included.

APPENDIX H. BS1499:PART 1: 1983

Reasonable freedom from stretcher strain can be achieved in skin passed material by effective roller levelling immediately prior to pressing at the customer's plant.

Complete freedom from stretcher strain and also freedom from deterioration in cuctility, due to strain-age-hardening, is achieved by the supply of skin passed, stabilized, steels as established by

Table 5. Chemical composition

Grade (see notes 2 and 3)	Rolled condition (see note 2 to 2.5)	Quality	C max.	Mn max.	S max.	p max,
-						10
1	HR, HS	Extra deep drawing	%	25	0%	% .
	CR, CS	aluminium-killed steel Extra deep drawing aluminium-	0.08	0.45	0.030	0.025
2	HR. HS, CR, CS	Killed stabilized steel	0.08	0.45	0.030	0.025
3	HR, HS, CR, CS	Extra deep drawing	0.08	0.45	0.035	0.030
4	HR, HS, CR, CS	Deep drawing	0.10	0.50	0.040	0.040
4	HR. HS.	Drawing or forming Flanging	0.12	0.60	0.050	0.050
5	HR. HS.	Commercial	0.15	0.60	0.050	0.050
See also sec		Commercial	0.20	0.90	0.050	0.060

NOTE 1. For improved atmospheric corrosion resistance, material can be supplied with a specified copper content

NOTE 2. Steels that have received a decarburizing treatment are not supplied against these grades unless previously

NOTE 3. Steels in this section may not be suitable for case-hardening (see grade 10 in table 15).

Table 6. Mechanical properties of hot rolled material (note 1)

Rolled condition	Yield strength,	Tensile strength,	Elongation	Elongation, A, min. (note 2) Original gauge length, Lo			Bend mandrel diameter (180 ° bend)		
	Re, min.	R _m , min.	Original				ben diameter (180 ben	id)	
				50 mm .	80 mm (note 4)	200 mm (note 3)	a < 3 mm	3 mm > a < 10 mm	a > 10 mm
HR1, HS1	N/mm ²	N/mm ²	%	%	%				
HR2, HS2	170	290	34	(32)	25	0a	0 <i>a</i>		
HR3; HS3	(170)	(290)	(28)	(26)	(21)	0a	0a	_	
HR4, HS4 HR14, HS14	(170)	(280)	(25)	(23)	(18)	1a	2 <i>a</i>	2	
HR15, HS15	(170)	(280)	-	_	_	2 <i>a</i>	3 <i>a</i>	3a 4a	

NOTE 1. The mechanical properties shown correspond to material in the despatched condition only. The strength will increase with cold forming. Tensile properties shown correspond to material in the despatched condition only. The strength will increase with cold forming. Tensile properties given in brackets are for guidance only and are not mandatory unless specially agreed at the time of ordering. Tensile test results are not normally requested for grades 3, 4, 14 and 15.

NOTE 2. For material of less than 2.5 mm thickness the percentage elongation is reduced by 1 for each 0.25 mm reduction

NOTE 3. Elongation values measured on 200 mm gauge length are applicable to material rolled on wide mills only.

NOTE 4. The 80 mm gauge length is currently not used in the UK but, as a step towards conforming with European

Table 7. Mechanical properties of cold rolled material produced on wide mills: ie. rolled in widths ≥ 600 mm) (note 1)

Rolled	Yield strength.	Tensile strength.	Elongation	, A, min. (r	note 2)	Bend mandrel	Modified
	R _e , min.			uge length,	Lo	(180° bend)	Erichsen cupping test
		50	50 mm	80 mm (note 3)	200 mm		
CR1 CR2 33 /	N/mm ² 140 • 140 (140) (140)	N/mm ² 280 280 (280) (280)	% 38 36 · (34)	% (36) (34) (32)	% 29 27 (25)	0a 0a 0a	See figure 3 for minimum values

s the thickness of the pend test diece.

DTE 1. Tensile properties given in prackets are for guidance only and are not mandatory unless.

Tensile rest results are not normally requested for grades. specially agreed at the time of ordering. Tensile test results are not normally requested for grades 3 and 4.

DTE 2. For material less than 1,00 mm thick, the percentage elongation is reduced by 1 for each 25 mm reduction in thickness.

NOTE 3. The 80 mm gauge length is currently not used in UK but, as a step towards conforming with uropean practice, tentative values have been included.

Table 8. Mechanical properties of cold rolled material produced on narrow mills .e. rolled in widths < 600 mm and thicknesses ≤ 3 mm) (see note 7)

Rolled andition and grade	Annealed (A) or skin passed (SP)	Hardness HV	Yield strength, Re, min.	Tensile strength,	Elongatio	Bend mandrel diameter (180° bend)		
	condition	max.	/ e,	R _m , min.	Original g	Original gauge length, Lo		
		5 and 6)	(note 2)	(note 2)	50 mm	80 mm (note 8)		
			N/mm²	N/mm ²	%	96		
CS1	A	95	140	270	38	(36)	Ga	
	SP	105	140	270	36	(34)	0a	
CS2	A	95	140	270	36	(34)	0a	
	SP	100	140	270	36	(34)	Ga Ga	
:S3	А	100	(140)	(280)	(34)	(32)	Ūa	
	SP	110	(140)	(280)	(34)	(32)	ūa.	
~S4	A	105	(140)	(280)	-	-	Oa .	
	SP	115	(140)	(280)	_		Ga	

a is the thickness of the bend test piece.

IOTE 1. Tensile properties given in brackets are for guidance only and are not mandatory unless specially greed at the time of ordering.

NOTE 2. Narrow strip is supplied to comply with either the hardness and bend tests or the tensile and bend tests but in no case with both the nargness and tensile tests.

IOTE 3. For material less than 1.0 mm thickness, the percentage elongation is reduced by 1 for each 25 mm reduction in thickness. Values should be agreed between the manufacturer and purchaser for thicknesses of less than 0.5 mm.

NOTE 4. The mechanical properties apply to material in the as-received condition only. The strength ill increase with cold forming. Due note should be made of any possibility of age hardening (see

NOTE 5. For condition SP, with plating finish (PF) or mirror finish (MF) or material which is supplied free from stretcher marks', the maximum hardness may be increased by 5 points HV or the tensile rength by 20 N/mm2

IOTE 6. The hardness values for grades CS2, CS3 and CS4 apply only to rimmed steels.

NOTE 7. For material with thickness exceeding 3 mm, the mechanical properties are to be agreed at

IOTE 8. The 80 mm gauge length is currently not used in the UK but, as a step towards conforming European practice, tentative values have been included.

Table 11. Mechanical properties: carbon-manganese steels

Grade	Roiled condition	Yield strength.	Tensile strength,	Elongation, A, min,		Elongation, A, min.		
	(see note 1)	R _e , min. R _m , min		Original	gauge length	. 40	diameter (180 ° bend)	
				50 mm	80 mm (note 4)	200 mm	2 and 3)	
34/20 37/23 43/25 50/35	HR. HS. CR. CS HR. HS. CR. CS HR. HS HR. HS	N/mm ² 200 230 250 350	N/mm ² 340 370 430 500	% 29 28 25 20	% (27) (26) (23) . (18)	% 21 20 16	2a 2a 3a	

a is the thickness of the bend test piece.

NOTE 1. The properties of HS materials are only applicable up to and including 8 mm, For material thicker than 8 mm, the properties are to be agreed between the manufacturer and purchaser.

NOTE 2. In the case of grades 34/20 and 37/23, for steel 3 mm thick and over, the bend test requirement is for a mandrel diameter of 2a. For special applications, these grades together with grades 43/25 and 50/35 may be ordered with a bend test requirement of a mandrel diameter of 2a.

NOTE 3. The cend test requirements quoted in this table are for specially prepared test pieces (see 1.10.2); conditions during fabrication may be more severe and not be simulated by those during laboratory testing (see note, "Manipulation", to section three and table 14),

NOTE 4. The 30 mm gauge length is currently not used in UK but, as a step towards conforming with European practice, tentative values have been included.

The symbols, if required, denoting material condition (see table 1 and note 2 to 3.5.1) shall be given before the grade number of the steel, in the following order:

- (a) the symbol R, B or K signifying the type of steel;
- (b) the symbols HR, HS, CR or CS, signifying the method of rolling.

NOTE 1. Attention is drawn to the fact that it is not obligatory for the purchaser of a fabricated component to specify the manufacturing method for the flat rolled product.

In this section the grade number shall indicate, respectively, minimum tensile strength/minimum yield strength in N/mm² x 10⁻¹ (see the example below). A letter F in place of the oblique line, e.g. 40F30, shall denote steels which offer superior formability for the same strength levels as the corresponding steels in the upper part of table 13.

Symbols denoting surface finish shall appear after the grade number of the steel.

Example. HR37/23P signifies a hot rolled wide material having a specified minimum tensile strength and yield strength of 370 N/mm² and 230 N/mm² respectively, supplied with a pickled finish.

NOTE 2. Conditions and surface finish. The following material conditions are svailable: more complete descriptions are given in table 1 and table 2:

- HR Hot roiled on wide mills. Also available pickled (P)
- HS Hot rouled on narrow mills. Also available pickled (P). CR Cold rolled on wide mills. Grades 34/20 and 37/23 are
- available as general purpose only (GP).

 CS Cold roiled on narrow mills. These steels are normally supplied with a bright finish (BR).

Table 12. Chemical composition: micro-alloyed steels

Grade (see note 1)	Roilea	C max.	Mn max.	S max.	p max.
		0%	%	9/0	96
40/30	HR, HS, CS	0.15	1.20	0.040	0.040
43/35	HR, HS, CS	0.15	1.20	0.040	0.040
46/40	HR, HS, CS	0.15	1.20	0.040	0.040
50/45	HR, HS, CS	0.20	1.50	9.040	0.040
60/55	- HS, CS	0.20	1.50	0.040	0.040
40F30	HR, HS, CS	0.12	1.20	0.030	0.030
43F35	HR, HS, CS	0.12	1.20	0.030	0.030
46F40	HR, HS, CS	0.12	1.20	0.030	0.030
50F45	HR. HS, CS	0.12	1.20	0.030	0.030
60F55	- HS, CS	0.12	1.20	0.030	0.030
68F62	- HS -	0.12	1.50	0.030	0.030
75F70	− HS	0.12	1.50	0.030	0.030

NOTE 1. These grades are fine-grained, fully killed steels containing additions of micro-alloying elements such as Nb and Ti. The manufacturer, at his option, may also add certain elements, e.g. Ca, Ce and Zr, in order to modify the shape of the sulphide inclusions to achieve the nigh degree of formability offered by these steels, particularly the F series (see note 2).

NOTE 2. The steels including ${\sf F}$ in their designation offer superior formability for the same strength levels as the corresponding steels in the upper part of the table.

NOTE 3. For improved atmospheric corrosion resistance, these grades may be supplied with a specified minimum copper content by agreement between manufacturer and purchaser.

a.5.2 Weldability. All the grades specified in tables 10 and 12 shall be weldable provided that the welding techniques employed make allowance for composition and thickness. See BS 693, BS 1140, BS 2630, BS 5135 and BS 6265.

3.5.3 Strain-age-embrittlement. Where proof of freedom from strain-age-embrittlement is required (see 3.2(g)), the method of test shall be agreed between the manufacturer and the purchaser, as the test defined in 1.11 may not be appropriate to all the steels in this section.

Table 13. Mechanical properties: micro-alloyed steels

Grade	Rolled condition (see note 1)	Yield strength, Re, min. (see note 2)	Tensile strength, R_m , min.	Elongation, A, min. Original gauge length, Lo			Bend mandrel diameter (180° bend)
						N/mm²	N/mm ²
40/30	HR, HS, CS	300	400	26	(24)	18	2a
43/35	HR, HS, CS	350	430	23	(21)	16	2 <i>a</i>
46/40	HR, HS, CS	400	460	20	(18)	12	3 <i>a</i>
50/45	HR, HS, CS	450	500	20	(18)	12	3a
60/55	-, HS, CS	550	600	17	(15)	10	3.5a
40F30	HR, HS, CS	300	400	28	(26)	20	0a
43F35	HR, HS, CS	350	430	25	(23)	18	0.5a
46F40	HR, HS, CS	400	460	22	(20)	14	1 <i>a</i>
50F45	HR, HS, CS	450	500	22	(20)	14	1.5a
60F55	-, HS, CS	550	600	19	(17)	11	1.5a
68F62	-, HS -	620	680	18	(16)	10	2 <i>a</i>
75F70	-, HS -	700	750	15	(13)	8	3 <i>a</i>

a is the thickness of the bend test piece.

NOTE 1. The properties of HS materials are only applicable up to and including 8 mm. For material thicker than 8 mm, the properties are to be agreed between the manufacturer and purchaser.

NOTE 2. A specific range for the yield strength of any particular grade and thickness may be agreed between manufacturer and purchaser at the time of ordering.

NOTE 3. The bend test requirements quoted in this table are for specially prepared test pieces (see 1.10); conditions during fabrication may be more severe and not simulate those during laboratory testing (see note, "Manipulation", to section three and table 14).

NOTE 4. The 80 mm gauge length is currently not used in the UK but, as a step towards conforming with European practice, tentative values have been included.